



CONSTRUCT

U.PORTO

FEUP FACULDADE DE ENGENHARIA
UNIVERSIDADE DO PORTO

Seismic behavior of precast piers on high speed railway bridges

André Monteiro

António Arêde

Nelson Vila Pouca

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1. PhD framework:

- Main work developed on the scope of the SIPAV project.
- Challenged proposed by *Mota-Engil, Betões e Prefabricados*:
 - Full-scale application viable for high-speed railway bridges (HSRL);
 - Precast solution for fast construction;
 - Reinforced concrete (RC) based layout;
 - Focus on the piers for seismic behavior assessment;
- State-of-art of relevant areas reviewed accordingly:

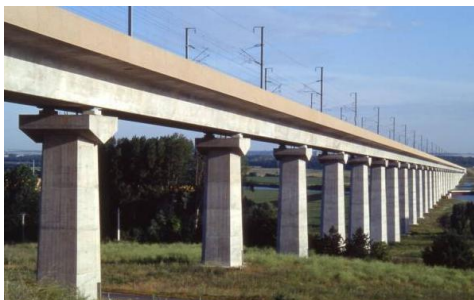
HSRL + Precast solution + RC + Piers = Virtually non-existent

- Opportunity to address the shortcoming by aiming to apply common precast solutions for HSRL Piers as well.

2. HSRL Bridge Piers:

- Common Layouts for HSRL Bridge piers:

Single Column Pier
(often with flare)



Tall viaducts

Wall Pier



Multiple Column Pier
(Bent-type column)

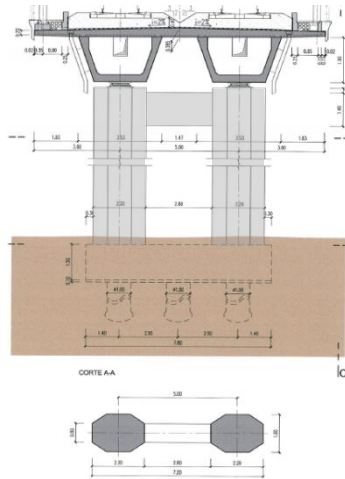


Long/short viaducts

- Increased stiffness relative to equivalent motorway bridge piers.
- Strict deformation limits for HSRL running safety limit state, e.g.:
 - Maximum radius for lateral deflection of 1.50×10^{-3} rad;
 - Maximum longitudinal displacement of 5.00 mm;
- High seismic forces are expected.

3. Base Structure for Study:

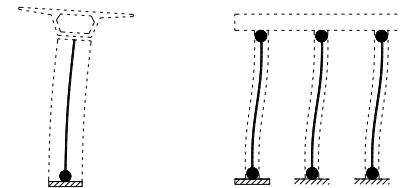
- Poceirão – Caia HSRL proposal for long and low height viaducts:



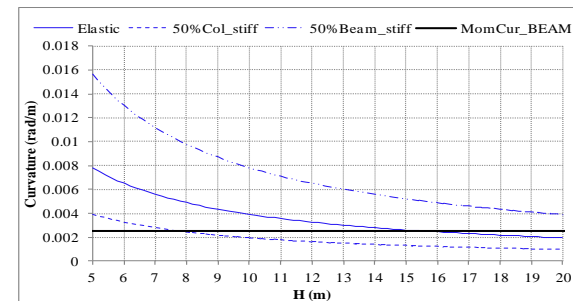
- Double Column RC Pier ($5.00\text{m} < H_{pier} < 20.00\text{m}$);
- Short span coupling beam ($\alpha_s = 1.0$);
- High stiffness columns;

Seismic design guidelines:

- Plastic hinges;
- High ductility;

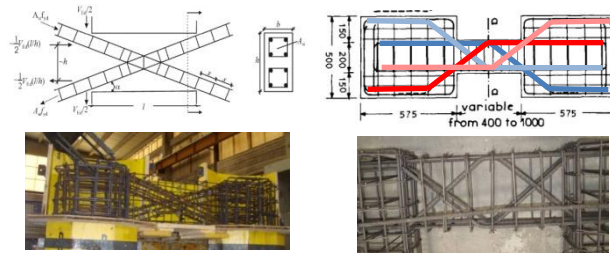


- **Height dependent** stiffness ratios lead to difficulty in evaluating suitable locations for inelastic deformations.
 - Possibility for **high shear ductility demand** in the beams.
-
- The graph plots Curvature (rad/m) on the y-axis (0 to 0.018) against Height H (m) on the x-axis (5 to 20). Four curves are shown: Elastic (solid blue), 50% Col_stiff (dashed blue), 50% Beam_stiff (dotted blue), and MomCur_BEAM (solid black). The Elastic curve starts at ~0.008 rad/m at H=5m and decreases to ~0.002 rad/m at H=20m. The 50% Col_stiff curve starts at ~0.016 rad/m at H=5m and decreases to ~0.004 rad/m at H=20m. The 50% Beam_stiff curve starts at ~0.016 rad/m at H=5m and decreases to ~0.001 rad/m at H=20m. The MomCur_BEAM curve is a constant horizontal line at ~0.002 rad/m.
- | H (m) | Elastic (rad/m) | 50% Col_stiff (rad/m) | 50% Beam_stiff (rad/m) | MomCur_BEAM (rad/m) |
|-------|-----------------|-----------------------|------------------------|---------------------|
| 5 | 0.008 | 0.016 | 0.016 | 0.002 |
| 10 | 0.004 | 0.008 | 0.006 | 0.002 |
| 15 | 0.003 | 0.005 | 0.003 | 0.002 |
| 20 | 0.002 | 0.004 | 0.001 | 0.002 |



4. Prototype Solutions:

- Beam reinforcement layouts based on applications for **coupling beams of shear walls**:

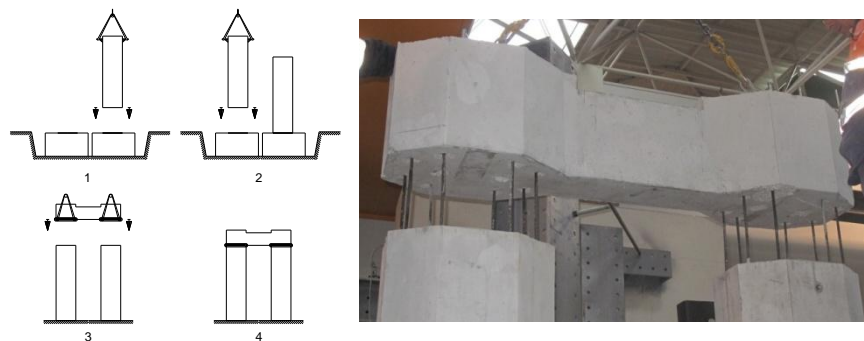


Bi-diagonal layout (Eurocode 8 / ACI318).

Rhombic truss (Tegos and Penelis (1988)).

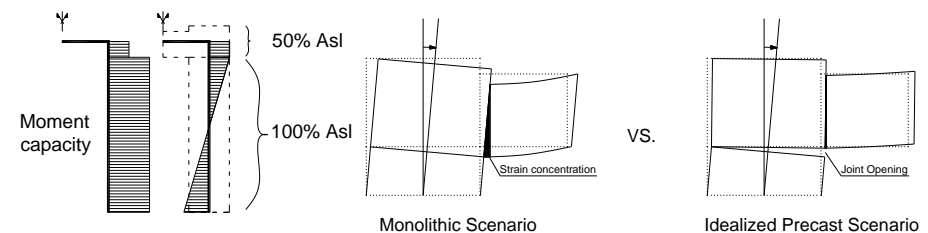
Dowel Rhombic truss (adapted from Tassios *et al.* (1996)).

- Precast system based on a top-down assembly (2 columns + beam), enforcing **reinforcement yielding at the joint**.



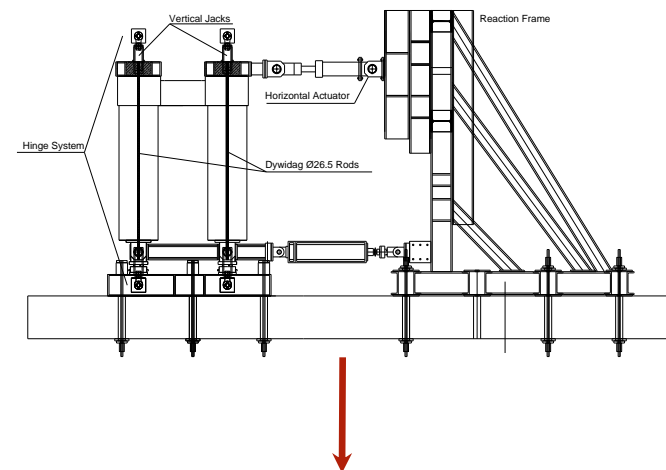
Assembly Procedure

Precast Joint Reinforcement Design:



5. Experimental Campaign

- Test setup designed and installed in LESE accounting for:
 - Constant axial loading on both columns (300 kN);
 - Free rotation on column bases;
 - Cyclic loading applied through shear;



Vertical Jacks



Threadbar Connection



Shear Loading Plates

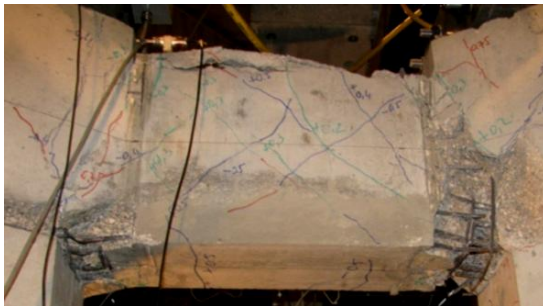


Rotation Hinge



6. Main Observations

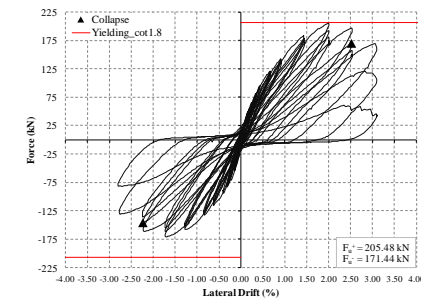
- Monolithic Specimens largely influenced by beam shear, providing generally **low ductility** capacity.



Sliding shear



Diagonal splitting

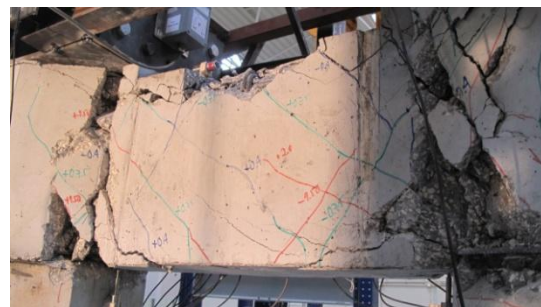


Low ductility and energy dissipation

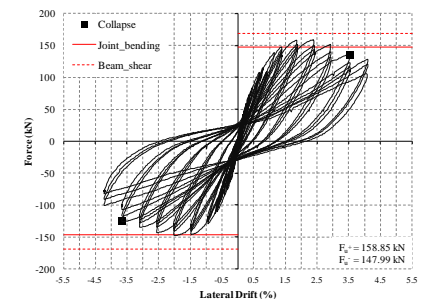
- Precast Specimens benefitting from the **flexibility** provided by the joint, although still subjected to heavy damage.



Large joint displacements



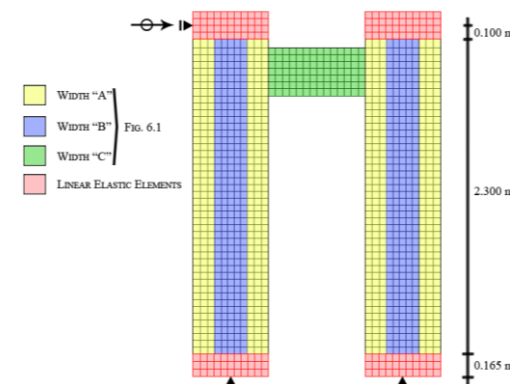
Heavy damage on larger drifts



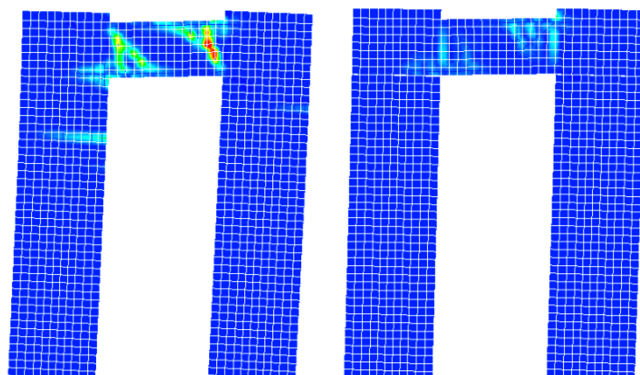
Increased ductility
and energy dissipation

7. Numerical Modelling

- 2D FEM based plane stress approach on Cast3m, with individual constitutive characterization of:
 - Concrete (Faria, R. and Oliver, J. (1993));
 - Steel reinforcement (Menegotto, M. and Pinto, P. (1973));
 - Bond-slip behavior (Eligehausen *et al.* (1982));
 - Joint behavior (Snyman *et al.* (1991));



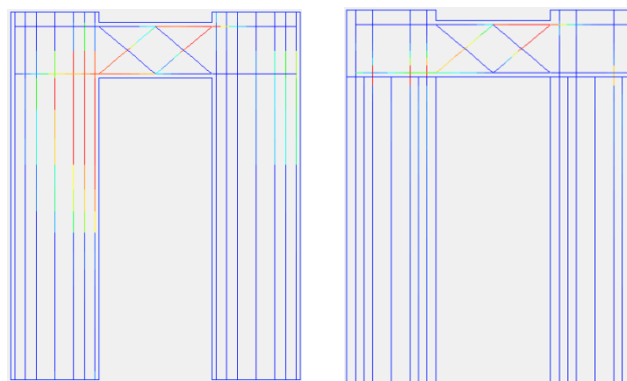
ϵ_{11} strains at first yielding:



Monolithic specimen

Precast specimen

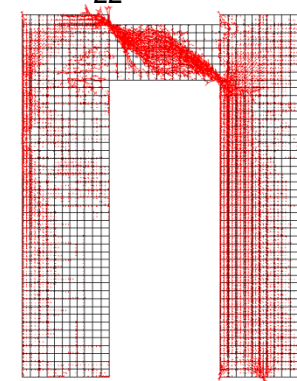
Rebar stresses:



Monolithic specimen

Precast specimen

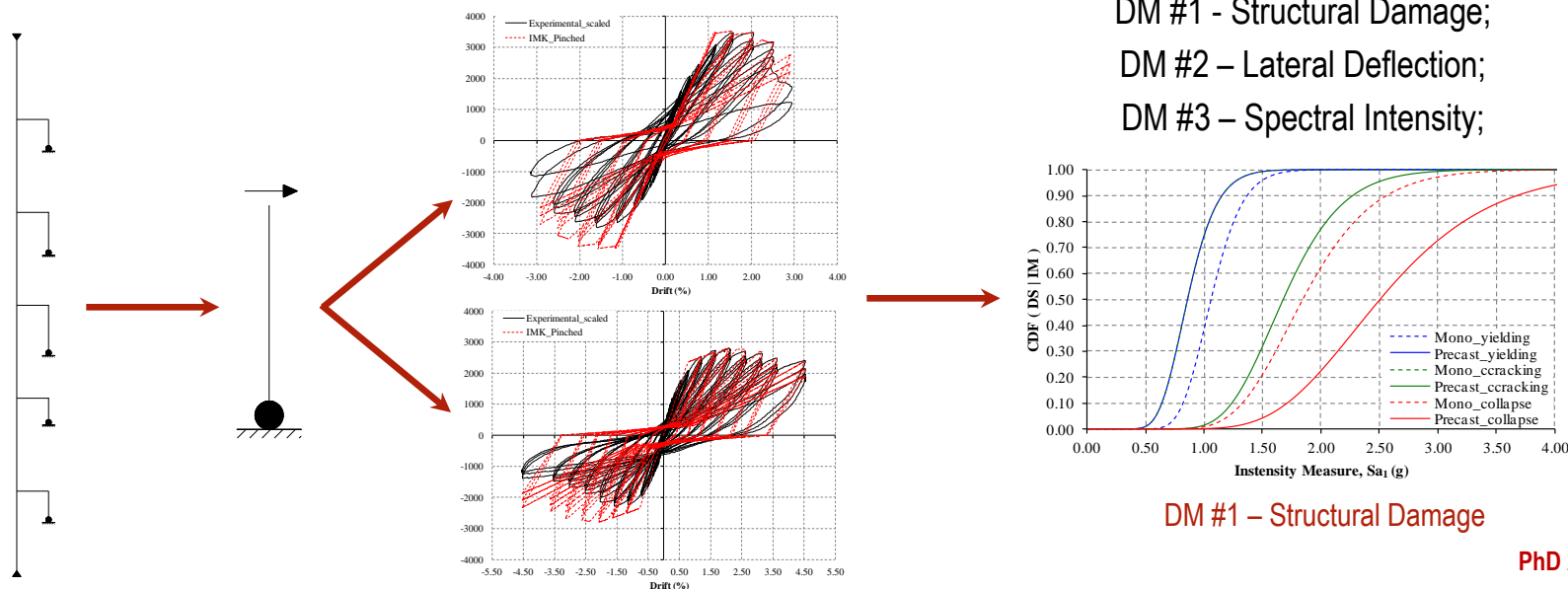
σ_{22} stresses:



Both specimens

8. Seismic Performance

- Experimental data used to calibrate global modelling tools for seismic performance assessment:
 - Viaduct modelling using **2D characterization** in OpenSees;
 - **Lumped plasticity** at the base of each pier calibrated accordingly, for Monolithic and Precast specimens (Ibarra *et al.* (2005));
 - **IDA** procedures (Vamvatsikos, D. and Cornell, C.A. (2002)); Comparison of different Damage Measures:





Conclusions

- A **double column bent pier** system was studied for precast application;
- There is a **high shear ductility demand** in the beam of the structural system, requiring **unconventional reinforcement** layouts;
- Experimental evidence showed that the precast system helps with **increasing overall ductility and energy dissipation**;
- Numerical analyses confirm that the precast system is able to **globally improve the seismic performance** of the studied viaducts;

Thank you!